

MICROCOPY RESOLUTION TEST CHART NATIONAL BUREAU OF STANDARDS-1963-A





US Army Corps of Engineers

The Hydrologic Engineering Center

### HEC-5Q:

## SYSTEM WATER QUALITY MODELING



UTIC FILE COPY

DISTRIBUTION STATEMENT A

Approved for public releases

Distribution Unlimited

Technical Paper No. 111

January 1986

Papers in this series have resulted from technical activities of the Hydrologic Engineering Center. Versions of some of these have been published in technical journals or in conference proceedings. The purpose of this series is to make the information available for use in the Center's training program and for distribution within the Corps of Engineers.

The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.

The contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.

HEC-5Q: System Water Quality Modeling\*

R. G. Willey 1

#### INTRODUCTION

Several state-of-the-art models [2,3,5] are available for analyzing water quality conditions in complex reservoir systems for a given set of operational conditions. Some of these models can even make operational decisions regarding proper gate regulations to obtain a desirable water quality condition at a dam site for a given set of flow conditions.

HEC-5Q, Simulation of Flood Control and Conservation Systems (Including Water Quality Analysis) [4] computer model, has the unique capabilities to accept user-specified water quantity and quality needs system-wide and to decide how to regulate the network of reservoirs. The decision criteria are programmed to consider flood control, hydropower, instream flow (municipal, industrial, irrigation, water supply, fish habitat) and water quality requirements.

The HEC-5Q program was first applied to the Sacramento River system in California and a report was published in July 1985 [8]. Two other applications are in progress, the Kanawha and Monongahela River systems, and are expected to be completed by September 1986. A brief description of the HEC-5Q concepts and these three applications will be discussed below.

#### MATHEMATICAL MODEL CONCEPTS

HEC-5Q has been developed specifically for evaluating the type of problem shown in Figure 1. The model is capable of evaluating a reservoir system of up to ten reservoirs and up to thirty control points. The model will define a best system operation for water quantity and quality; evaluating operational concerns like flood control, hydropower, water supply, and irrigation diversions. Since the computer program users manual [4], and several technical papers [1,6,7] adequately document the details of the model concepts and the input description, only a brief overview is provided below.

<sup>\*</sup> Presented at the U.S. Army Corps of Engineers 1986 Water Quality Seminar, New Orleans, Louisiana, February 1986.

Hydraulic Engineer, U.S. Army Corps of Engineers, Hydrologic Engineering Center, 609 Second Street, Davis, CA 95616

#### Flow Simulation Module

The flow simulation module was developed to assist planning studies for evaluating proposed reservoirs in a system and to assist in sizing the flood control and conservation storage requirements for each project recommended for the system. program can be used to show the effects of existing proposed reservoirs on flows and damages in a complex reservoir The program can also be system. in selecting the proper reservoir releases throughout the system to minimize flooding as possible as while maintaining a balance of flood control storage ("balanced pool") among the reservoirs.

#### Water Quality Simulation Module

The water quality simulation module is capable of analyzing water temperature and up to three conservative and three conservative constituents. If at least one of the nonconservative constituents is an oxygen demanding parameter. dissolved oxygen can also be analyzed.

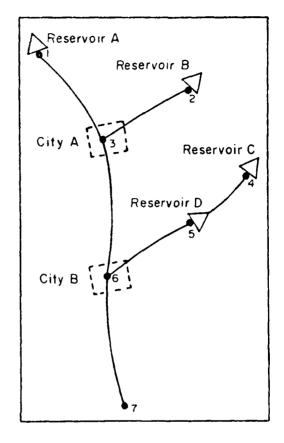


Figure I TYPICAL RESERVOIR SYSTEM SCHEMATIC

The water quality simulation module accepts system flows generated by the flow simulation module and computes the distribution of all the water quality constituents in up to ten reservoirs and their associated downstream reaches. The ten reservoirs may be in any configuration.

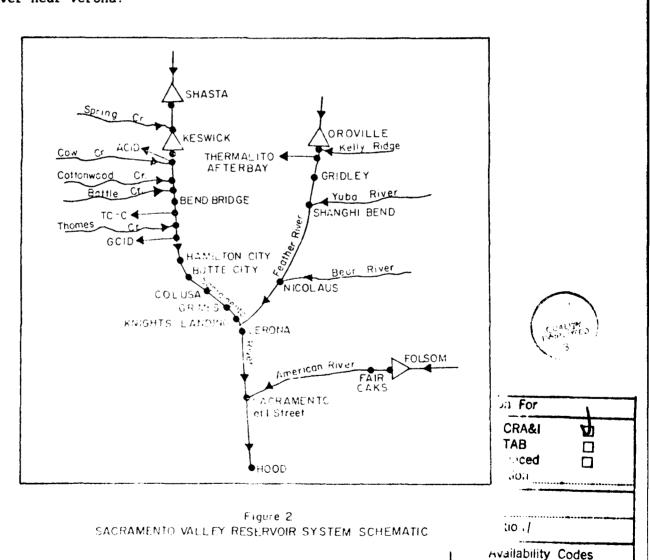
Gate openings in reservoir multilevel withdrawal structures are selected to meet user-specified water quality objectives at downstream control points. If the objectives cannot be satisfied with the previously computed "balanced pool" flows, the model will compute a modified flow distribution necessary to better satisfy all downstream objectives. With these capabilities, the planner may evaluate the effects on water quality of proposed reservoir-stream system modifications and determine how a reservoir intake structure should be operated to achieve desired water quality objectives within the system.

#### SACRAMENTO RIVER SYSTEM APPLICATION

The Sacramento Valley reservoir system consists of four major reservoirs as shown in Figure 2. Shasta and Keswick Reservoirs are located on the Sacramento River in northern California about 240 miles north of Sacramento. Below Shasta and above Keswick, inter-basin water transfers enter the Sacramento River through Spring Creek. Along the Sacramento River, Cow Creek and Cottonwood Creek are major inflowing tributaries and the Anderson-Cottonwood (ACID), Tehama-Colusa (TC), Corning (C) and Glenn-Colusa (GCID) Irrigation District Canals are major irrigation diversions.

The state of the s

Oroville Reservoir is located on the Feather River in the Sierra foothills about 100 miles north of Sacramento. Major tributaries entering the Feather River include the Yuba and Bear Rivers. Major diversions are located immediately below Oroville Dam from the Thermalito Afterbay. The Feather River flows into the Sacramento River near Verona.



3

Avail and for

Special

Dist

Folsom Reservoir is located on the American River in the Sierra foothills about 30 miles east of Sacramento. The American River below Folsom Reservoir is leveed with no major tributaries entering before its confluence with the Sacramento River at Sacramento.

The Sacramento River continues to flow south towards the San Francisco Bay. This study's lower boundary is located near Hood about 20 miles south of Sacramento.

The application of the HEC-5Q model to the Sacramento Valley reservoir system, includes data ^ssembly, model execution and interpretation of results as described below and elsewhere in more detail [8].

#### Data Assembly

The HEC-5Q model data requirements are similar to those of most comprehensive water quality models. The data to be assembled are categorized into three types: time independent, required time dependent and optional time dependent.

The time independent data include: physical description of the reservoir (i.e., elevation, volume, surface area, discharge capacity, and vertical reservoir segmentation), physical description of the river (i.e., cross sections, channel discharge capacity, and river reach segmentation), control point desired and required flows, model coefficients (i.e., flow routing; reservoir diffusion; physical, chemical and biological reaction rates) and initial conditions for the start of the simulation. The required time dependent data include: evaporation, meteorology, diversions, inflow quantity and quality for all reservoir and river tributaries, discharge quantity from reservoirs (only required to reproduce historical operation), and control point target flow and water quality conditions. optional time dependent data include: reservoir storages; river flows at other than control points; and reservoir and river water quality profiles. These data are used as checks on the model output in contrast to the previously mentioned data which are required to make the model work.

Sources for the data categorized above are numerous. In general, they include all water-related agencies at the federal, state, local and private levels. Meteorological data are readily available from the U.S. Weather Service, local airports and universities. The primary data source is the NOAA's National Weather Service (NWS) office in Asheville, North Carolina.

Tributary inflows, diversions and reservoir discharges may be readily available from WATSTORE and STORET data systems. WATSTORE is managed by the USGS and contains streamflow data. STORET is managed by the EPA and contains water quality data. These computer data systems can often provide the necessary tributary inflow quantity and quality data.

#### Model Execution

The model simulation for the Sacramento Valley system used temperature, specific conductance (sometimes called electrical conductivity), alkalinity, carbonaceous biochemical oxygen demand (BOD), ammonia (NH3) and dissolved oxygen (DO). These specific parameters were chosen based on the availability of at least limited data.

The model can be used for existing and/or proposed reservoirs. If an existing condition is being simulated, usually the objective is to reproduce historical events through model calibration. Selection of the calibration option can significantly decrease computer time by not using the time-consuming linear and non-linear programming algorithms in the model.

Once the model has been calibrated, the objective may be to modify an existing reservoir operation pattern or to evaluate the impact of proposed new reservoirs or channel modifications. This analysis requires the use of the linear and non-linear programming algorithms. These algorithms compute the water quality targets at the dam which will best meet all the user-specified downstream targets and decide on the best gate operations to meet these computed targets.

The simulation mode discussed above can be used either to evaluate the best water quality that can be provided throughout the system for given reservoir discharges (obtained either external to the simulation or determined by the HEC-5 flow simulation module) or to evaluate the best water quality operation without prespecified discharge quantities. The former operation is referred to as a balanced pool operation and the latter as a flow augmentation operation.

When using the balanced pool operation, the HEC-5Q program simply evaluates the best vertical level for withdrawal (assuming multiple level intakes are available) at each reservoir to meet all downstream water quality targets for the given reservoir discharge determined by the flow simulation module.

The flow augmentation operation allows the model to relax the balanced pool concept and to decide how much flow should come from which reservoir and at which vertical level in order to meet downstream water quality targets. Sometimes downstream water quality improvements require significantly increased discharge rates to obtain only small improvements in water quality. This flow augmentation operation is the most time consuming mode of execution.

For the Sacramento River application, the input data set was executed using the calibration option. Application of this option allows the user to define the exact level of the intake structure operated. This is the normal method of model application when calibrating the model to observed historical data.

#### Interpretation of Results

The HEC-5Q execution of the Sacramento Valley reservoir system produced results which were compared to observed water quantity and quality data in the four reservoirs and at all downstream control points. The data for comparison purposes consisted of discharge rates at most control points as well as water temperature at many of the same locations. Other water quality parameters are less available but were compared where they were available. Selected portions of the graphical display of these results are shown in Figures 3-6 for the reservoirs and at selected locations along the stream network.

These plots satisfactorily demonstrate the capability of HEC-5Q to reasonably reproduce observed reservoir and stream profiles on large systems. The legend on the reservoir temperature graph defines simulated and observed data for various dates. Shasta, Oroville and Folsom Reservoirs have sufficient observed temperature data to be useful for calibration purposes. Sufficient observed data for the other parameters were not available. (Only data for Shasta Reservoir are shown due to space limitations.) Considering the model limitation (at the time of application, but since corrected) of having only one weather station for the entire system, it is the authors' opinion that the reproduction is quite good. Perhaps some further refinement could be achieved with additional trials but the acceptability of the model can be demonstrated with these results.

The legend on the stream plots defines the various observed and simulated water quality parameters for the study period. Simulated constituents 1 and 2 are specific conductance (or EC) and alkalinity. Unlike the simulated data, the observed data points are often more than one day apart. Some caution should be applied to interpretation of the connecting line between observed data points further apart than one or two days.

In general, the calibration of the model is quite good along the Sacramento River for all the observed parameters down to Hamilton City, inclusive. (Only data for Hamilton City are shown due to space limitations.) Butte City and Colusa measured temperatures show that significant warming of this reach of the Sacramento River takes place, at least during the Spring (April and May 1956). This temperature increase, in addition to the lack of sufficient simulated quantity of flow at Butte City and Colusa (compared to accurate simulation of flow at Bend Bridge), suggests that the undefined return flows on the Sacramento River between Hamilton City and Knights Landing are significant and need to be evaluated.

The Feather River below Oroville and the American River below Folsom lack sufficient water quality data to provide adequate information for calibration purposes.

Since the Sacramento River below Sacramento is the combined product of all three river systems, the inaccuracies already discussed are also apparent at this location. Careful interpretation and

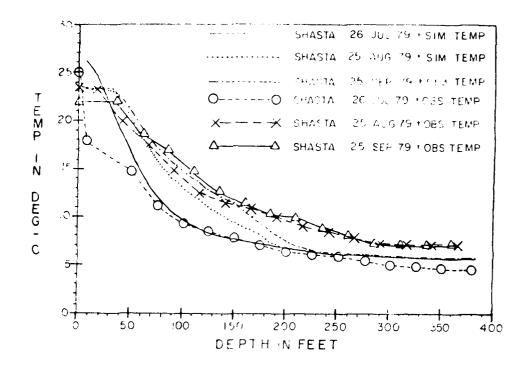


Figure 3
SHASTA RESERVOIR TEMPERATURE PROFILES

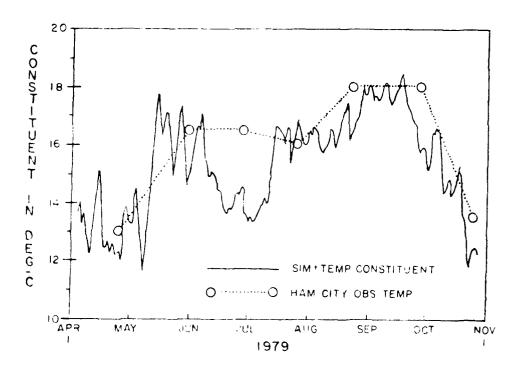


Figure 4
SACRAMENTO RIVER AT HAMILTON CITY - WATER TEMPERATURE

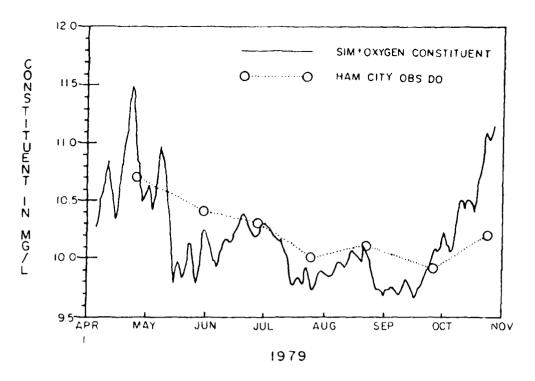


Figure 5

#### SACRAMENTO RIVER AT HAMILTON CITY-DISSOLVED OXYGEN

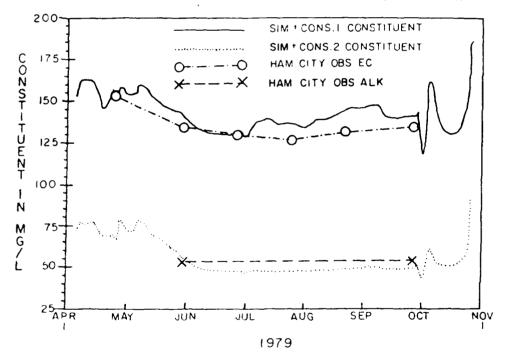


Figure 6

SACRAMENTO RIVER AT HAMILTON CITY - SPECIFIC

CONDUCTANCE (EC) & ALKALINITY

evaluation of all these results lead the authors to encourage the continued application of this model to help develop understanding of the workings and operation of any stream system.

#### KANAWHA RIVER SYSTEM APPLICATION

The Kanawha River system consists of three major reservoirs as shown in Figure 7. Bluestone Lake Dam on the New River is located about 107 miles south of Charleston, West Virginia. The Greenbrier River, a significant tributary, drains into the New River immediately below Bluestone Lake Dam and above Hinton, West Virginia; the site of significant gaged flow and water quality data. The Summersville Lake Dam on the Gauley River flows into the New River at Kanawha Falls, about 40 miles above Charleston. The Sutton Lake Dam on the Elk River flows into the Kanawha River at Charleston. The New River is renamed the Kanawha River at Kanawha Falls.

The Kanawha River system is being evaluated using HEC-5Q for temperature, specific conductance (or electrical conductance, EC) biochemical oxygen demand (BOD) and dissolved oxygen (DO). Since the necessary input data are available at Hinton (below Bluestone Lake) and analysis of Bluestone Lake is of no interest, only the two lakes, Summersville and Sutton, are being analyzed along with the three-river network.

The study is still in progress. Only initial calibration efforts have been completed. This study will be the second test case of real prototype data and has already helped debug several parts of the code not previously tested. The case is similar to the Sacramento River case but involves only Corps reservoirs; therefore, it is a likely candidate for future model testing in the real-time water control and reservoir operation area.

#### MONONGAHELA RIVER SYSTEM APPLICATION

The Monongahela River system consists of three major reservoirs as shown in Figure 8. Stonewall Jackson Lake Dam (presently under construction) is located about 202 miles south of Pittsburgh, Pennsylvania, on the West Fork Monongahela River. Tygart Lake Dam about 152 miles north of Pittsburgh on the Tygart River drains into the Monongahela River at Fairmont, West Virginia (CP25 in Figure 8). Youghiogheny Lake Dam about 85 miles east of Pittsburgh on the Youghiogheny River drains into the Monongahela River at Braddock, West Virginia (CP65). Braddock is about 11 miles south of Pittsburgh.

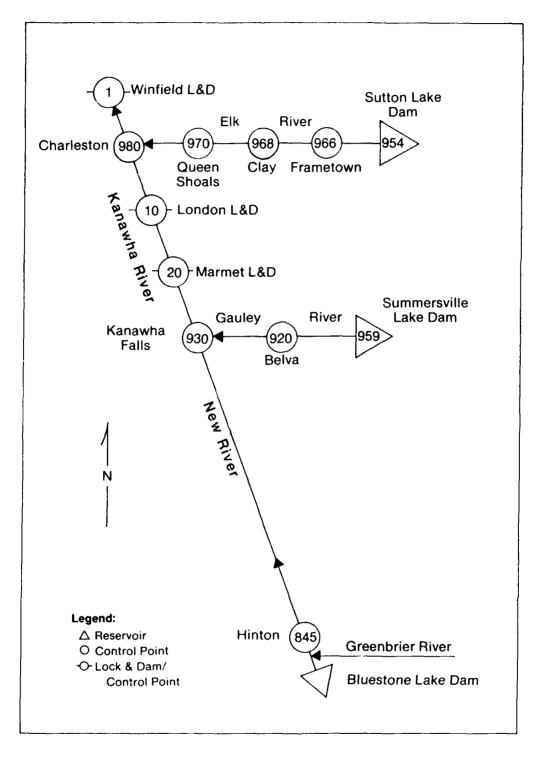


Figure 7
KANAWHA RIVER BASIN SCHEMATIC

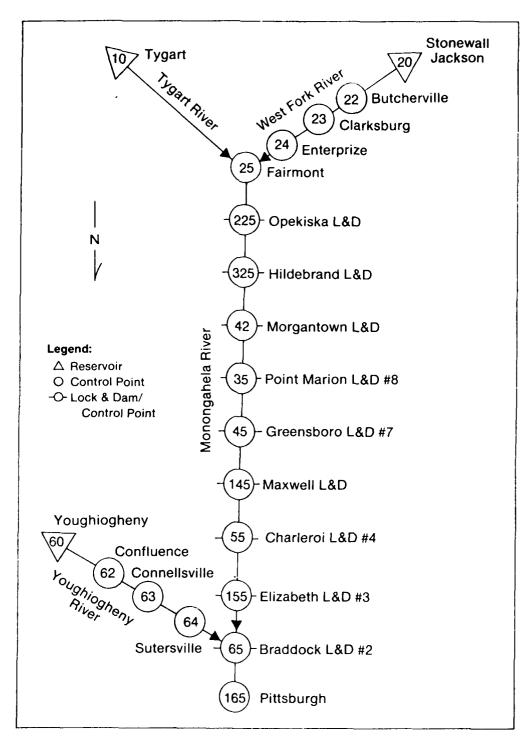


Figure 8

MONONGAHELA RIVER BASIN SCHEMATIC

The Monongahela River system is being evaluated using HEC-5Q for temperature, EC, BOD, and DO. The study is also still in progress with data files still being developed. This study will be the third test case and, similar to the Kanawha River system, it is a candidate for future testing of real-time operations capability. This data set could easily be modified to include evaluation of hydropower retrofit on the nine locks and dams along the lower Monongahela River up to Fairmont.

#### SUMMARY

In this paper, the author has provided a brief description of the HEC-5Q computer program for analysis of water quality impacts due to reservoir system operations and a discussion of three applications, one completed and two in progress. The model results are very encouraging and future applications are being considered.

The model has the capability to evaluate present operations on large integrated reservoir systems such as the Columbia/Snake/Williamette Rivers or similar large systems in other parts of the United States. Once calibrated to historical conditions, alternative regulation can be easily evaluated to "best" meet all project purposes at all points in the system and provide the water managers with input to their operation decisions either in a planning or "real-time" mode.

#### REFERENCES

- Duke, James H., Donald J. Smith and R.G. Willey, 1984, "Reservoir System Analysis for Water Quality," Technical Paper No. 99, Hydrologic Engineering Center.
- 2. Hydrocomp, 1976, "Hydrocomp Simulation Programming Operations Manual," 4th Edition, Palo Alto, California.
- 3. Hydrologic Engineering Center, 1978, "Water Quality for River-Reservoir Systems," Computer Program Description.
- 4. Hydrologic Engineering Center, 1984, "HEC-5Q, Simulation of Flood Control and Conservation Systems (Including Water Quality Analysis)," Draft Computer Program Users Manual.
- 5. U.S. Army Engineer Waterways Experiment Station, 1982, "CE-QUAL-R1: A Numerical One-Dimensional Model of Reservoir Water Quality," Instruction Report E-82-1, Computer Program User's Manual.
- 6. Willey, R.G., 1983, "Reservoir System Regulation for Water Quality Control," Technical Paper No. 88, Hydrologic Engineering Center.
- Willey, R.G., D.J. Smith J.H. Duke, 1985, "Modeling Water Resources Systems for Water Quality," Technical Paper No. 104, Hydrologic Engineering Center.
- 8. Willey, R.G., 1985, "Water Quality Simulation of Reservoir System Operations in the Sacramento Valley Using HEC-5Q," Training Document No. 24, Hydrologic Engineering Center.

#### TECHNICAL PAPERS (TP)

Technical papers are written by the staff of the HEC, sometimes in collaboration with persons from other organizations, for presentation at various conferences, meetings, seminars and other professional gatherings.

This listing includes publications starting in 1978.

HEC NUMBER	TITLE	HEC <u>PRICE</u>	ntis <u>Number</u>
		\$2.00 Eac	<u>h</u>
TP-52	Potential Use of Digital Computer Ground Water Models, D. L. Gundlach, Apr 78, 38 pp.		AD-A106 251
TP-53	Development of Generalized Free Surface Flow Models Using Finite Element Techniques, D. M. Gee and R. C. MacArthur, Jul 78, 21 pp.		AD-A106 252
TP-54	Adjustment of Peak Discharge Rates for Urbanization, D. L. Gundlach, Sep 78, 7 pp.		AD-A106 253
TP-55	The Development and Servicing of Spatial Data Management Techniques in the Corps of Engineers, R. P. Webb and D. W. Davis, Jul 78, 26 pp.		AD-A106 254
TP-56	Experiences of the Hydrologic Engineering Center in Maintaining Widely Used Hydrologic and Water Resource Computer Models, B. S. Eichert, Nov 78, 16 pp.		AD-A106 255
TP-57	Flood Damage Assessments Using Spatial Data Management Techniques, D. W. Dav and R. P. Webb, May 78, 27 pp.	is	AD-A106 256
TP-58	A Model for Evaluating Runoff-Quality in Metropolitan Master Planning, L. A. Roesner, H. M. Nichandros, R. P. Shubinski, A. D. Feldman, J. W. Abbott, and A. O. Friedland, Apr 72, 81 pp.		AD-A106 257

#### TECHNICAL PAPERS (TP) (Continued)

HEC NUMBER	TITLE	HEC PRICE	NTIS NUMBER
		\$2.00 Eac	<u>h</u>
TP-59	Testing of Several Runoff Models on an Urban Watershed, J. Abbott, Oct 78, 53 pp.		AD-A106 258
TP-60	Operational Simulation of a Reservoir System with Pumped Storage, G. F. McMahon, V. R. Bonner and B. S. Eichert, Feb 79, 32 pp.		AD-A106 259
TP-61	Technical Factors in Small Hydropower Planning, D. W. Davis, Feb 79, 35 pp.		AD-A109 757
TP-62	Flood Hydrograph and Peak Flow Frequency Analysis, A. D. Feldman, Mar 79 21 pp.		AD-A109 758
TP-63	HEC Contribution to Reservoir System Operation, B. S. Eichert and V. R. Bonner, Aug 79, 28 pp.		AD-A109 759
TP-64	Determining Peak-Discharge Frequencies in an Urbanizing Watershed: A Case Study S. F. Daly and J. C. Peters, Jul 79, 1		AD-A109 760
TP-65	Feasibility Analysis in Small Hydropower Planning, D. W. Davis and B. W. Smith, Aug 79, 20 pp.		AD-A109 761
TP-66	Reservoir Storage Determination by Computer Simulation of Flood Control and Conservation Systems, B. S. Eichert, Oct 79, 10 pp.		AD-A109 762
TP-67	Hydrologic Land Use Classification Using LANDSAT, R. J. Cermak, A. D. Feldman and R. P. Webb, Oct 79, 26 pp.		AD-A109 763
TP-68	Interactive Nonstructural Flood-Control Plannng, D. T. Ford, Jun 80, 12 pp.		AD-A109 764

ACCOUNT OF THE PROPERTY OF THE

#### TECHNICAL PAPERS (TP)(Continued)

HEC NUMBER	TITLE	HEC PRICE	NTIS NUMBER
		\$2.00 Eac	<u>h</u>
TP-69	Critical Water Surface by Minimum Specific Energy Using the Parabolic Method, B. S. Eichert, 1969, 15 pp.		AD-A951 599
TP-70	Corps of Engineers Experience with Automatic Calibration of a Precipitation-Runoff Model, D. T. Ford E. C. Morris, and A. D. Feldman, May 80, 12 pp.	ı <b>,</b>	AD-A109 765
TP-71	Determination of Land Use from Satellite Imagery for Input to Hydrologic Models R. P. Webb, R. Cermak, and A. D. Feldm Apr 80, 18 pp.		AD-A109 766
TP-72	Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flo and Water Quality, R. C. MacArthur and W. R. Norton, May 80, 12 pp.	W	AD-A109 767
TP-73	Flood Mitigation Planning Using HEC-SAM, D. W. Davis, Jun 80, 17 pp.		AD-A109 756
TP-74	Hydrographs by Single Linear Reservoir Model, J. T. Pederson, J. C. Peters, and O. J. Helweg, May 80, 17 pp.		AD-A109 768
TP-75	HEC Activities in Reservoir Analysis, V. R. Bonner, Jun 80, 10 pp.		AD-A109 769
TP~76	Institutional Support of Water Resource Models, J. C. Peters, May 80, 23 pp.		AD-A109 770
TP-77	Investigation of Soil Conservation Service Urban Hydrology Techniques, D. G. Altman, W. H. Espey, Jr. and A. D. Feldman, May 80, 14 pp.		AD-A109 771
TP-78	Potential for Increasing the Output of Existing Hydroelectric Plants, D. W. Davis and J. J. Buckley, Jun 81, 20 pp.		AD-A109 772

#### TECHNICAL PAPERS (TP) (Continued)

HEC <u>Number</u>	TITLE	HEC PRICE	NTIS NUMBER	
		\$2.00 Eac	<u>h</u>	
TP-79	Potential Energy and Capacity Gains from Flood Control Storage Reallocation at Existing U. S. Hydropower Reservoirs, B. S. Eichert and V. R. Bonner, Jun 81, 18 pp.		AD-A109 78	87
TP-80	Use of Non-Sequential Techniques in the Analysis of Power Potential at Storage Projects, G. M. Franc, Jun 81, 18 pp.	e	AD-A109 78	88
TP-81	Data Management Systems for Water Resources Planning, D. W. Davis, Aug 81, 12 pp.	3	AD-A114 65	50
TP-82	The New HEC-1 Flood Hydrograph Package, A. Feldman, P. B. Ely and D. M. Goldman, May 81, 28 pp.	D.	AD-A114 36	60
TP-83	River and Reservoir Systems Water Quality Modeling Capability, R. G. Willey, Apr 82, 15 pp.		AD-A114 19	92
TP-84	Generalized Real-Time Flood Control System Model, B. S. Eichert and A. F. Pabst, Apr 82, 18 pp.		AD-A114 35	5 <b>9</b>
TP-85	Operation Policy Analysis: Sam Rayburn Reservoir, D. T. Ford, R. Garland and C. Sullivan, Oct 81, 16 pp.		AD-A123 52	26
TP-86	Training the Practitioner: The Hydrologic Engineering Center Program, W. K. Johnson, Oct 81, 20 pp.		AD-A123 56	<b>68</b>
TP-87	Documentation Needs for Water Resources Models, W. K. Johnson, Aug 82, 16 pp.		AD-A123 55	58
TP-88	Reservoir System Regulation for Water Quality Control, R.G. Willey, Mar 83, 18 pp.		AD-A130 82	29
TP-89	A Software System to Aid in Making Real-Tim Water Control Decisions, A. F. Pabst and J. C. Peters, Sep 83, 17 pp.	ne	AD-A138 61	<b>i</b> 6

#### TECHNICAL PAPERS (TP) (Continued)

HEC Number	TITLE	HEC PRICE	NTIS NUMBER
		\$2.00 Each	<u>1</u>
TP-90	Calibration, Verification and Application of a Two-Dimensional Flow Model, D. M. Gee, Sep 83, 6 pp.		AD-A135 668
TP-91	HEC Software Development and Support, B. S. Eichert, Nov 83, 12 pp.		AD-A139 009
TP-92	Hydrologic Engineering Center Planning Models D. T. Ford and D. W. Davis, Dec 83, 17 pp.		AD-A139 010
TP-93	Flood Routing Through a Flat, Complex Floodplain Using A One-Dimensional Unsteady Flow Computer Program, J. C. Peters, Dec 83, 8 pp.		AD-A139 011
TP-94	Dredged-Material Disposal Management Model, D. T. Ford, Jan 84, 18 pp.		AD-A139 008
TP-95	Inflitration and Soil Moisture Redistribution in HEC-1, A. D. Feldman, Jan 84,	ion	AD-A141 626
TP-96	The Hydrologic Engineering Center Experience in Nonstructural Planning, W. K. Johns and D. W. Davis, Feb 84, 7 pp.		AD-A141 860
TP-97	Prediction of the Effects of a Flood Contro Project on a Meandering Stream, D. M. Gee, Mar 84, 12 pp.	<b>)</b> 1	AD-A141 951
TP-98	Evolution in Computer Programs Causes Evolution in Training Needs: The Hydrologic Engineering Center Experience, V. R. F. Jul 84, 20 pp.		AD-A145 601
TP-99	Reservoir System Analysis for Water Quality J. H. Duke, D. J. Smith and R. G. Will Aug 84, 27 pp.	, ,	AD-A145 680

#### TECHNICAL PAPERS (TP)(Continued)

HEC NUMBER	TITLE	HEC PRICE	NTIS NUMBER
		\$2.00 Eac	<u>:h</u>
TP-100	Probable Maximum Flood Estimation - Easter United States, P. B. Ely and J. C. Pe Jun 84, 5 pp.		AD-A146 536
TP-101	Use of Computer Program HEC-5 For Water Supply Analysis, R. J. Hayes and Bill S. Eichert, Aug 84, 7 pp.		AD-A146 535
TP-102	Role of Calibration in the Application of HEC-6, D. Michael Gee, Dec 84, 19 pp.		AD-A149 269
TP-103	Engineering and Economic Considerations in Formulating Nonstructural Plans, M. W. Burnham, Jan 85, 16 pp.		A150 154
TP-104	Modeling Water Resources Systems for Water Quality, R. G. Willey, D. J. Smith and J. H. Duke, Feb 85, 10 pp.		AD-A154 288
TP-105	Use of a Two-Dimensional Flow Model to Quantify Aquatic Habitat, D. M. Gee and D. B. Wilcox, Apr 85, 10 pp.		AD-A154 287
TP-106	Flood-Runoff Forecasting with HEC1F, J. C. Peters and P. B. Ely, May 85, 7 pp.		AD-A154 286
TP-107	Dredged-Material Disposal System Capacity Expansion, D. T. Ford, Aug 85, 23 pp.		
TP-108	Role of Small Computers in Two-Dimensional Flow Modeling, D. M. Gee, Oct 85, 6 pp	· .	AD-A159 666
TP-109	One Dimensional Model For Mud Flows, D. R. Schamber and R. C. MacArthur, Oct 85, 6 pp.		AD-A159 921
TP-110	Subdivision Froude Number, David H. Schoellhamer, John C. Peters, and Bruce E. Larock, Oct 85, 6 pp.	,	AD-A160 486
TP-111	HEC-5Q: System Water Quality Modeling, R. G. Willey, Jan 86, 10 pp.		

SECURITY CLASSIFICATION OF THIS DAGE (When Data Entered)	
REPORT DOCUMENTATION PAGE	READ INSTRUCTIONS BEFORE COMPLETING FORM
Technical Paper No. 111 ADA 164832	3. RECIPIENT'S CATALOG NUMBER
4. TITLE (and Subtitle)	5. TYPE OF REPORT & PERIOD COVERED
HEC-50: System Water Quality Modeling	6. PERFORMING ORG. REPORT NUMBER
7. AUTHOR(a)	8. CONTRACT OR GRANT NUMBER(e)
R. G. Willey	
9. PERFORMING ORGANIZATION NAME AND ADDRESS U.S. Army Corps of Engineers The Hydrologic Engineering Center 609 Second Street, Davis, California 95616	10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS
11. CONTROLLING OFFICE NAME AND ADDRESS	January 1986
	13. NUMBER OF PAGES
14. MONITORING AGENCY NAME & ADDRESS(if different from Controlling Office)	15. SECURITY CLASS. (of this report)
	15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
16. DISTRIBUTION STATEMENT (of this Report)	
Distribution is unlimited.	

17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)

#### 18. SUPPLEMENTARY NOTES

Presented at the U.S. Army Corps of Engineers 1986 Water Quality Seminar, New Orleans, Louisiana, February 1986.

19. KEY WORDS (Continue on reverse side if necessary and identify by block number)

SReservoir System Analysis, Hater Quality, Case Study, Stream System Analysis, Computer Model

#### 20. ABSTRACT (Continue on reverse side if necessary and identify by block number)

>> Several state-of-the art models are available for analyzing water quality conditions in complex reservoir systems for a given set of operational conditions. Some of these models can even make operational decisions regarding proper gate regulations to obtain a desirable water quality condition at a dam site for a given set of flow conditions.

HEC-5Q, Simulation of Flood Control and Conservation Systems (Including Water Quality Analysis) computer model, has the unique capabilities to accept SECURITY CLASSIFICATION OF THIS PAGE(When Data Entered) user-specified water quantity and quality needs system-wide and to decide how to regulate the network of reservoirs. The decision criteria are programmed to consider flood control, hydropower, instream flow (municipal, industrial, irrigation, water supply, fish habitat) and water quality requirements. The HEC-50 program was first applied to the Sacramento River system in California and a report was published in July 1985. Two other applications are in progress, the Kanawha and Monongahela River systems, and are expected to be completed by September 1986. A brief description of the HEC-50 concepts and these three applications are discussed.

UNCLASSIFIED

# DTIC END) 4-86